

# **Equivalent Framing Products Catalog Table of Contents**

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AWCI | ASSOCIATION OF THE WALL AND CEILING INDUSTRY



### **Product Certifications**

Studs Unlimited products are manufactured to meet or exceed all applicable standards including:

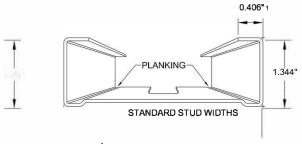
- AISI S100-2016 North American Specification for the Design of Cold-Formed Steel Structural Members.
- IBC 2018 International Building Code 2018.
- AISI S220-2015 North American Standard for Cold-Formed Steel Framing Nonstructural Members.
- **ASTM A1003** Standard Specification for Steel Sheet, Carbon, Metallic and Nonmetallic Coated for Cold Formed Framing Members.
- **ASTM A653** Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process.

- **ASTM C645** Standard Specification for Nonstructural Steel Framing Members.
- **ASTM C754** Standard Specification for Installation of Steel Framing Members to Receive Screw-Attached Gypsum Panel Products.
- **ASTM E72** Standard Test Methods of Conducting Strength Tests of Panels for Building Construction.
- **ASTM E90** Standard Test Method for Laboratory Measurement of Airborne Sound Transmission Loss of Building Partitions and Elements.
- ICC ES Evaluation Report ESR-4205
- CCRR-0224 (through association with SFIA)
- UL Fire Listed through the SFIA

Studs Unimited is a proud member of the **SFIA** (**Steel Framing Industry Association**) and the products included in the catalog are certified by the SFIA to ensure consistent quality and compliance to all applicable industry standards via unannounced 3<sup>rd</sup> party independent testing.

#### Framing Stud Profiles

#### PrimeWall® 20 EQ Stud



10.25" RETURN ON 1.625" STUDS



# **Comparison of Equivalent Products**

### **Comparison Chart of Significant EQ Technical Data**

		20 ga Equival	lent Products	
Manufacturer	Studs Unlimite	ed EQ Products	ClarkDietrich*	Marino\Ware**
Product Description		PrimeWall ®	ProStud® 20	ViperStud® 20
Minimum Thickness in MILS		19/21	18	18
Yield Strength - KSI		55	70	70
Ratio of KSI to Thickness <sup>2</sup>		2.90 / 2.62	3.89	3.89
Limiting Heights:				
Non-Composite - Fully Braced -				
5 PSF, L/240, 16" Spacing:				
162xxxxxx		7' 9"	7' 0"	6' 11"
250xxxxxx		10'7"	9' 11"	9' 8"
362xxxxxx		14' 0"	13' 2"	12' 10"
400xxxxxx		15' 2"	14' 1" <sup>1</sup>	13' 10"
600xxxxxx		18' 11" <sup>1</sup>	18' 9" <sup>1</sup>	18' 6"
Composite - 5 PSF, L/240, 16" Spacing:				
162xxxxxx			11' 1"	10' 0"
250xxxxxx		401.4411	14' 0"	13' 1"
362xxxxxx 400xxxxxx		16' 11"	16' 10" 17' 7"	16' 4" 17' 3"
600xxxxxx		24' 3"	23' 8"	23' 7"
Allowable Ceiling Spans:		210	20 0	20 7
Unsupported Joist Spacing -				
4 PSF, L/240, 16" Spacing:				
162xxxxxx		7' 9"	7' 3"	7' 3"
250xxxxxx		9' 2"	8' 5"	8' 1"
362xxxxxx		9'11"	9' 2"	8' 11"
400xxxxxx		10' 2"	9' 5"	9' 2"
600xxxxxx		11' 10"	10' 11"	10' 4"
Torsional Section Properties:				
Jx1000 (in <sup>4</sup> ):				
162xxxxxx		0.016	0.010	
250xxxxxx 362xxxxxx		0.015 0.018	0.013 0.015	
400xxxxxx		0.019	0.016	
600xxxxx		0.033	0.021	
Lu (in):				
162xxxxxx		25.1	24.8	21.2
250xxxxxx		27.6	24.5	21.9
362xxxxxx		26.9	24.3	21.5
400xxxxxx		26.8 20.5	24.2	21.5
600xxxxxx		29.5	23.6	21.5

<sup>1</sup> Web-height to thickness ratio exceeds 200. Web stiffeners are required.

ProStud® is a registered trademark of ClarkDietrich Building Systems. ViperStud® is a registered trademark of Ware Industries, Inc.

<sup>2</sup> Studs Unlimited believes the ratio of KSI to Thickness is an important indicator of screw acceptance. The lower the ratio the better, especially in thinner gauge products.

<sup>\*</sup>Information obtained from the ClarkDietrich catalog effective 9/23/2020.\*\* Information obtained from the Marino\Ware catalog effective 10/1/2020.



# PrimeWall® 20 EQ (19/21mil)

### **General 19/21mil EQ Product Information**

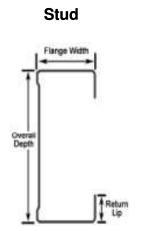
The PrimeWall® 20 EQ studs are used in non-load-bearing wall systems that support gypsum board construction. PrimeWall® 20 EQ studs are equivalent to 20 gauge (30mil) standard non-structural drywall studs.

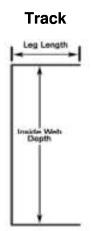
The PrimeWall® stud is manufactured with web planking.

PrimeWall® framing products have a G40 or a minimum standard coating that complies with the requirements of IBC 2018 and ASTM A1003/ A1003M. G60 and G90 coatings are available upon request.

PrimeWall® framing products are produced with steel that meets or exceeds a 55 KSI yield strength. The 1-5/8" and 6" stud products have a design thickness of 0.0221" (21mil). All other stud and track products have a design thickness of 0.020" (19mil).

All products comply with the manufacturing tolerances listed in ASTM C645. (Table 1 Manufacturing Tolerances for non-structural members).





#### Advantages of this Product:

- 14' limiting height in non-composite wall applications highest in class.
- 16' 11" limiting height in composite wall applications highest in class.
- Longer leg 1.34" for larger area for fastening.
- Lower KSI than competitors this helps with better screw acceptance.
- Consumes 35% less steel than standard products.

#### PrimeWall® 20 EQ Stud

- Nominal Web/Depth: 1-5/8", 2-1/2", 3-5/8", 4", 6"
- Flange Width: 1-11/32"
- Return/Lip: 13/32" (1/4" on 1-5/8" studs)
- Design Thickness: 0.020" (2-1/2", 3-5/8", 4" studs) and 0.0221" (6" and 1-5/8" studs)

#### PrimeWall®20 EQ Track

- Nominal Web/Depth: 1-5/8", 2-1/2", 3-5/8", 4", 6"
- Leg Length: 1-1/4", 1-1/2", 2", 3"
- Design Thickness: 0.020"



# PrimeWall® 20 EQ 19/21mil)

# **General Product Information**

PrimeWall® 20 E	Q (19/	21mil) E	Orywall Fr	aming Stu	ıd						
		Materia	l Requiremen	its			Product Me	asurements			
	Yield	Mil	Design	Base Metal	Web	nominal	Le	е	Retur	n/Li	Color-
PrimeWall Stud		Thickness	Thickness	Thickness	Decimal	Fraction	Decimal	Fraction	Decimal	Fraction	Coding
162PWS134-21	55	21	0.0221	0.0210	1.625"	1-5/8"	1.344"	1 -11/32"	0.250"	1/4"	Brown
250PWS134-19	55	19	0.0200	0.0190	2.5"	2-1/2"	1.344"	1 -11/32"	0.406"	13/32"	Brown
362PWS134-19	55	19	0.0200	0.0190	3.625"	3-5/8"	1.344"	1 -11/32"	0.406"	13/32"	Brown
400PWS134-19	55	19	0.0200	0.0190	4.0"	4"	1.344"	1 -11/32"	0.406"	13/32"	Brown
600PWS134-21	55	21	0.0221	0.0210	6.0"	6"	1.344"	1 -11/32"	0.406"	13/32"	Brown

### PrimeWall® 20 EQ (19/21mil) -- Drywall Framing Track

		Materia	I Requiremen	ts		Product Me	asurements	3	
	Yield	Mil	Design	<b>Base Metal</b>	Web (	nominal)	Leg / I	Flange	Color-
PrimeWall Track	(KSI)	Thickness	Thickness	Thickness	Decimal	Fraction	Decimal	Fraction	Coding
162PWT125-19	55	19	0.0200	0.0190	1.625"	1-5/8"	1.25"	1-1/4"	Brown
162PWT150-19	55	19	0.0200	0.0190	1.625"	1-5/8"	1.50"	1-1/2"	Brown
162PWT200-19	55	19	0.0200	0.0190	1.625"	1-5/8"	2.00"	2"	Brown
250PWT125-19	55	19	0.0200	0.0190	2.5"	2-1/2"	1.25"	1-1/4"	Brown
250PWT150-19	55	19	0.0200	0.0190	2.5"	2-1/2"	1.50"	1-1/2"	Brown
250PWT200-19	55	19	0.0200	0.0190	2.5"	2-1/2"	2.00"	2"	Brown
250PWT300-19	55	19	0.0200	0.0190	2.5"	2-1/2"	3.00"	3"	Brown
362PWT125-19	55	19	0.0200	0.0190	3.625"	3-5/8"	1.25"	1-1/4"	Brown
362PWT150-19	55	19	0.0200	0.0190	3.625"	3-5/8"	1.50"	1-1/2"	Brown
362PWT200-19	55	19	0.0200	0.0190	3.625"	3-5/8"	2.00"	2"	Brown
362PWT300-19	55	19	0.0200	0.0190	3.625"	3-5/8"	3.00"	3"	Brown
400PWT125-19	55	19	0.0200	0.0190	4.0"	4"	1.25"	1-1/4"	Brown
400PWT150-19	55	19	0.0200	0.0190	4.0"	4"	1.50"	1-1/2"	Brown
400PWT200-19	55	19	0.0200	0.0190	4.0"	4"	2.00"	2"	Brown
400PWT300-19	55	19	0.0200	0.0190	4.0"	4"	3.00"	3"	Brown
600PWT125-19	55	19	0.0200	0.0190	6.0"	6"	1.25"	1-1/4"	Brown
600PWT150-19	55	19	0.0200	0.0190	6.0"	6"	1.50"	1-1/2"	Brown
600PWT200-19	55	19	0.0200	0.0190	6.0"	6"	2.00"	2"	Brown
600PWT300-19	55	19	0.0200	0.0190	6.0"	6"	3.00"	3"	Brown



# **Section Properties**

### Section Properties -- PrimeWall ® 20 EQ (19/21mil) Drywall Framing Stud

	Design					Gros	s Prope	rties			Ef	fective	Propertion	es			To	rsional	Properti	ies		
	Thickness	$F_y$	Area	Weight	l <sub>xx</sub>	$S_{xx}$	$R_x$	$I_{vy}$	$R_y$	l <sub>xx</sub>	$S_{xx}$	$M_{a-L}$	$M_{a-D}$	$V_{ag}$	$V_{aNet}$	Jx1000	C <sub>w</sub>	X <sub>o</sub>	m	R <sub>o</sub>	ß	Lu
Section	(in)	(ksi)	(in²)	(lb/ft)	(in⁴)	(in3)	(in)	(in⁴)	(in)	(in⁴)	(in³)	(in-k)	(in=k)	( <b>l</b> b)	( <b>l</b> b)	(in⁴)	(in <sup>6</sup> )	(in)	(in)	(in)		(in)
162PWS134-21	0.0221	55	0.101	0.34	0.048	0.059	0.688	0.024	0.491	0.043	0.037	1.22	1.17	539	150	0.016	0.015	-1.167	0.669	1.441	0.344	25.1
250PWS134-19	0.0200	55	0.115	0.39	0,119	0.095	1.017	0.031	0.515	0.111	0.075	2.48	2.02	311	222	0.015	0.047	-1.152	0.685	1,621	0.495	27.6
362PWS134-19	0.0200	55	0.138	0.47	0.280	0.155	1.427	0.035	0.502	0.269	0.097	3.20	3.02	208	185	0.018	0.100	-1.013	0.622	1.821	0.690	26.9
400PWS134-19	0.0200	55	0.145	0.49	0.353	0.176	1,559	0.036	0.497	0,340	0.108	3,54	3,35	188	188	0.019	0.123	0.975	0.603	1,905	0.738	26.8
600PWS134-21 <sup>1</sup>	0.0221	55	0.204	0.70	1.020	0.340	2,233	0.044	0.465	0.689	0.136	4.49	5.94	133	133	0.033	0.327	-0.813	0.523	2,422	0.857	29.5

### Section Properties -- PrimeWall ® 20 EQ (19mil) Drywall Framing Track

	Design			Gross Properties						Ef	fective l	Propertie	es		То	rsional	Properti	es	
	Thickness	$F_{y}$	Area	Weight	l <sub>xx</sub>	$S_{xx}$	$R_x$	$I_{vy}$	$R_y$	l <sub>xx</sub>	$S_{xx}$	$M_{a-L}$	$V_{ag}$	Jx1000	$C_{w}$	$X_{o}$	m	$R_o$	ß
Section	(in)	(ksi)	(in²)	(lb/ft)	(in⁴)		(in)	(in⁴)	(in)	(in⁴)	(in³)	(in-k)	(lb)	(in⁴)	(in <sup>6</sup> )	(in)		(in)	
162PWT125-19	0.0200	55	0.082	0.28	0.043	0.050	0.725	0.014	0.411	0.029	0.024	0.80	442	0.011	0.007	-0.877	0.504	1.210	0.475
162PWT150-19	0.0200	55	0.092	0.31	0.051	0.059	0.740	0.023	0.496					0.012	0.012	-1.106	0.626	1.420	0.394
162PWT200-19	0.0200	55	0.112	0.38	0.065	0.076	0.761	0.049	0.662					0.015	0.026	-1.575	0.871	1.870	0.291
250PWT125-19	0.0200	55	0.100	0.34	0.109	0.084	1.045	0.016	0.399	0.079	0.038	1.25	292	0.013	0.019	-0.768	0.460	1.357	0.680
250PWT150-19	0.0200	55	0.110	0.37	0.126	0.097	1.069	0.026	0.488					0.015	0.031	-0.982	0.578	1,531	0.589
250PWT200-19	0.0200	55	0.130	0.44	0.159	0.122	1.106	0.057	0.661					0.017	0.067	-1.428	0.818	1.923	0.449
250PWT300-19	0.0200	55	0.170	0.58	0.226	0.174	1.152	0.168	0.995					0.023	0.204	-2.359	1.307	2.807	0.294
362PWT125-19	0.0200	55	0.122	0.42	0.251	0.135	1.431	0.018	0.380	0.186	0.055	1.81	200	0.016	0.044	-0.665	0.414	1.623	0.832
362PWT150-19	0.0200	55	0.132	0.45	0.285	0.153	1.467	0.029	0.470					0.018	0.071	-0.862	0.527	1.765	0.762
362PWT200-19	0.0200	55	0.152	0.52	0.354	0.190	1.523	0.064	0.647					0.020	0.154	-1.279	0.759	2.091	0.626
362PWT300-19	0.0200	55	0.192	0.66	0.491	0.264	1.597	0.190	0.994					0.026	0.463	-2.168	1,239	2.870	0.430
400PWT125-19	0.0200	55	0.130	0.44	0.315	0.153	1.556	0.018	0.374	0.226	0.061	2.00	181	0.017	0.055	-0.637	0.400	1.722	0.863
400PWT150-19	0.0200	55	0.140	0.48	0.356	0.174	1.595	0.030	0.463					0.019	0.089	-0.829	0.512	1.856	0.801
400PWT200-19	0.0200	55	0.160	0.54	0.439	0.214	1.657	0.066	0.642					0.021	0.193	-1.237	0.742	2.165	0.674
400PWT300-19	0.0200	55	0.200	0.68	0.606	0.296	1.741	0.197	0.991					0.027	0.577	-2.112	1.218	2.911	0.474
600PWT125-191	0.0200	55	0.170	0.58	0.822	0.269	2.199	0.020	0.342					0.023	0.138	-0.522	0.341	2.285	0.948
600PWT150-191	0.0200	55	0.180	0.61	0.914	0.300	2.254	0.033	0.429					0.024	0.226	-0.690	0.443	2.396	0.917
600PWT200-191	0.0200	55	0.200	0.68	1.099	0.360	2.344	0.074	0.607					0.027	0.488	-1.055	0.659	2.641	0.840
600PWT300-191	0.0200	55	0.240	0.82	1.469	0.482	2.474	0.223	0.964					0.032	1.449	-1.862	1.117	3.243	0.670

#### <u>Sections Properties Notes:</u>

- 1. Section properties are in accordance with AISI S100-16 "North American Specification for the Design of Cold-Formed Steel Structural Members".
- 2. Effective properties include the strength increase from cold-work of forming per AISI S100 section A3.3.2 where applicable.
- 3. For deflection determination, use the effective moment of inertia. Effective moment of inertia is based on the effective width method of AISI S100, Appendix 1.
- 4. The effective moment of inertia for deflection is calculated at a stress which results in a section modulus such that the stress times the section modulus at that stress is equal to the allowable local buckling moment, Ma-L.
- 5. Tabulated gross properties are based on the full, unreduced section away from punchouts.
- 6. Effective X-X Axis properties of all stud and joist sections based on punched sections. Track sections are considered unpunched.
- 7. In Section Property Tables, allowable moment and shear do NOT include the 0.9 factor on  $\Omega$  per AISI S220-11.
- 8. Where effective properties are not listed for a section, web depth-to-thickness or flange width-to-thickness limits from the AISI S100 are exceeded. Only gross properties are available.
- 9. Web depth for track sections is equal to the nominal stud width plus 2 times the design thickness plus bend radius. Hems on nonstructural track sections are ignored.
- 10. All track properties based on unpunched sections.

<sup>1</sup> Web-height to thickness ratio exceeds 200. Web stiffeners are required at all support points and concentrated loads,



# **Limiting Wall Heights**

# **All Products Table Information**Limiting Heights Tables

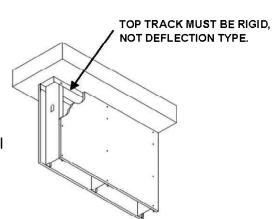
### Which Limiting Heights Table is the Correct Table to Use?

### **Composite Assemblies**

When the gypsum board is installed on both sides of the stud flange for the full height of the wall, it is a composite wall.

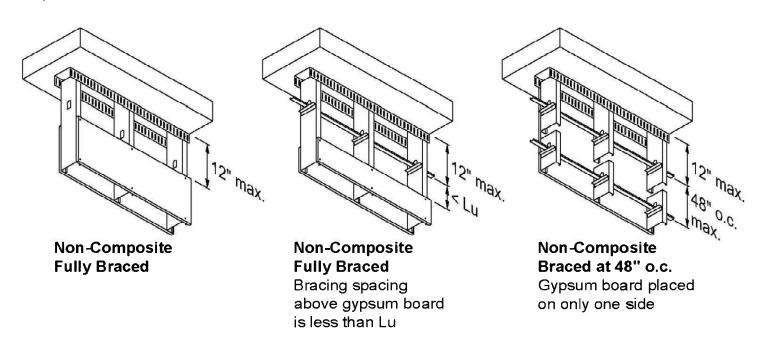
#### Composite

Gypsum board full height on both sides and fastened to all framing members, including top and bottom tracks.



### **Non-Composite Assemblies**

When the gypsum board stops at the ceiling level, but the stud continues to the deck, it is a non-composite condition.



Distance of unbraced length (Lu) can be found in the Section Properties starting on page 13.



# **Limiting Wall Heights – Non-Composite**

### Non-Composite Limiting Heights --Fully Braced --PrimeWall® 20 EQ (19/21mil)

	$F_{v}$	Spacing		5 psf			7.5 psf			10 psf	
Member	(ksi)	o.c. (in)	L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
162PWS134-21	55	12	10' 8"	8' 7"	7' 6"	9' 2"	7' 5"	6' 6"	8' 4"	6' 9"	5' 11"
162PWS134-21	55	16	9'7"	7' 9"	6' 9"	8' 4"	6' 9"	5' 11"	7' 6"	6' 2"	5' 5"
162PWS134-21	55	24	8'4"	6' 9"	5' 11"	7' 2"	5' 11"	5' 2"	6' 3"	5' 4"	4' 8"
250PWS134-19	55	12	14' 6"	11'7"	10' 2"	12'7"	10' 2"	8' 10"	11' 5"	9' 2"	8' 1"
250PWS134-19	55	16	13' 2"	10' 7"	9'3"	11'5"	9' 2"	8' 1"	10' 1"	8' 4"	7' 4"
250PWS134-19	55	24	11'5"	9' 2"	8' 1"	9' 6"	8' 0"	7' 0"	8' 2"	7' 3"	6' 5"
362PWS134-19	55	12	19'3"	15' 5"	13' 6"	16' 4"	13' 6"	11'9"	14' 2"	12' 3"	10' 8"
362PWS134-19	55	16	17' 4"	14' 0"	12'3"	14' 2"	12' 3"	10' 8"	12' 3"	11'1"	9' 9"
362PWS134-19	55	24	14' 2"	12' 3"	10' 8"	11'7"	10' 8"	9'4"	10' 0" <sup>e</sup>	9' 8" <sup>e</sup>	8' 6"
400PWS134-19	55	12	20' 10"	16' 8"	14'7"	17' 3"	14' 7"	12'9"	14' 11"	13' 2"	11'7"
400PWS134-19	55	16	18'4"	15' 2"	13' 3"	14' 11"	13' 2"	11'7"	12' 11"	11' 11"	10' 6"
400PWS134-19	55	24	14' 11"	13' 2"	11'7"	12' 2" <sup>e</sup>	11' 6"	10' 1"	10' 7" <sup>e</sup>	10' 5" <sup>e</sup>	9' 2" <sup>e</sup>
600PWS134-21	55	12	24' 5" <sup>e</sup>	20' 10" <sup>e</sup>	18' 2" <sup>e</sup>	20' 0" <sup>e</sup>	18' 2" <sup>e</sup>	15' 11" <sup>e</sup>	17' 3" <sup>e</sup>	16' 6" <sup>e</sup>	14' 5" <sup>e</sup>
600PWS134-21	55	16	21' 2" <sup>e</sup>	18' 11" <sup>e</sup>	16' 6" <sup>e</sup>	17' 3" <sup>e</sup>	16' 6" <sup>e</sup>	14' 5" <sup>e</sup>	15' 0" <sup>e</sup>	15' 0" <sup>e</sup>	13' 1" <sup>e</sup>
600PWS134-21	55	24	17' 3" <sup>e</sup>	16' 6" <sup>e</sup>	14' 5" <sup>e</sup>	14' 1" <sup>e</sup>	14' 1" <sup>e</sup>	12' 7" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	11' 6" <sup>e</sup>

- 1. Lateral loads have not been modified for deflection checks.
- 2. Loads or safety factors have not been modified for strength checks.
- 3. "Fully Braced" flexural strength taken as the minimum of local buckling and distortional buckling allowable moments.
- 4. For distortional buckling allowable moment, kφ = 0.
- 5. Moment of inertia for deflection is calculated at the maximum service level stress for the height listed. Note that this value may be higher than the effective lxx listed in section property tables.
- 6. Limiting heights are based on steel properties only.
- 7. Web crippling check based on 1 inch end bearing.
- 8. Shear and web crippling capacity have not been reduced for punchouts.
- 9. Limiting non-composite heights noted as "FULLY BRACED" based on continuous support of each flange over the full length of the stud.

<sup>&</sup>lt;sup>e</sup> Web Stiffeners are required at all support points and concentrated loads.



# **Limiting Wall Heights – Non-Composite**

### Non-Composite Limiting Heights -- Braced at 48" o.c. -- PrimeWall® 20 EQ (19/21mil)

	$F_{y}$	Spacing		5 psf			7.5 psf			10 psf	
Member	(ksi)	o.c. (in)	L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
162PWS134-21	55	12	10' 8"	8' 7"	7' 6"	9' 2"	7' 5"	6' 6"	8' 1"	6' 9"	5' 11"
162PWS134-21	55	16	9' 7"	7' 9"	6' 9"	8' 1"	6' 9"	5' 11"	7' 0"	6' 2"	5' 5"
162PWS134-21	55	24	8' 1"	6' 9"	5' 11"	6' 7"	5' 11"	5' 2"	5' 8"	5' 4"	4' 8"
250PWS134-19	55	12	14' 6"	11' 7"	10' 2"	12' 7"	10' 2"	8' 10"	11'5"	9' 2"	8' 1"
250PWS134-19	55	16	13' 2"	10' 7"	9' 3"	11' 5"	9' 2"	8' 1"	10'1"	8' 4"	7' 4"
250PWS134-19	55	24	11' 5"	9' 2"	8' 1"	9' 6"	8' 0"	7' 0"	8' 2"	7' 3"	6' 5"
362PWS134-19	55	12	18' 9"	15' 5"	13'6"	15' 4"	13' 6"	11' 9"	13' 3"	12'3"	10' 8"
362PWS134-19	55	16	16' 3"	14' 0"	12'3"	13' 3"	12' 3"	10' 8"	11' 6"	11'1"	9' 9"
362PWS134-19	55	24	13' 3"	12' 3"	10' 8"	10' 10"	10' 8"	9' 4"	9' 5" e	9' 5" e	8' 6"
400PWS134-19	55	12	19' 9"	16' 8"	14'7"	16' 1"	14' 7"	12' 9"	13' 11"	13' 2"	11' 7"
400PWS134-19	55	16	17' 1"	15' 2"	13'3"	13' 11"	13' 2"	11'7"	12'1"	11'11"	10' 6"
400PWS134-19	55	24	13' 11"	13' 2"	11'7"	11' 5"	11' 5"	10' 1"	9' 10" e	9' 10" e	9' 2" e
600PWS134-21	55	12	21' 10" <sup>e</sup>	20' 10" <sup>e</sup>	18' 2" <sup>e</sup>	17' 10" <sup>e</sup>	17' 10" <sup>e</sup>	15' 11" <sup>e</sup>	15' 5" <sup>e</sup>	15' 5" <sup>e</sup>	14' 5" <sup>e</sup>
600PWS134-21	55	16	18' 11" <sup>e</sup>	18' 11" <sup>e</sup>	16' 6" <sup>e</sup>	15' 5" <sup>e</sup>	15' 5" <sup>e</sup>	14' 5" <sup>e</sup>	13' 5" <sup>e</sup>	13' 5" <sup>e</sup>	13' 1" <sup>e</sup>
600PWS134-21	55	24	15' 5" <sup>e</sup>	15' 5" <sup>e</sup>	14' 5" <sup>e</sup>	12' 7" <sup>e</sup>	12' 7" <sup>e</sup>	12' 7" <sup>e</sup>	10' 11" <sup>e</sup>	10' 11" <sup>e</sup>	10' 11" <sup>e</sup>

- 1. Limiting heights are in accordance with AISI S100-16.
- 2. Lateral loads have not been modified for deflection checks.
- 3. Loads or safety factors have not been modified for strength checks.
- 4. 48" and 72" o.c. braced flexural strength taken as the minimum allowable moment based on local buckling, distortional buckling and lateral-torsional buckling with an unbraced length of 48 or 72 inches.
- 5. For distortional buckling allowable moment,  $k\phi = 0$ .
- 6. Moment of inertia for deflection is calculated at the maximum service level stress for the height listed. Note that this value may be higher than the effective lxx listed in section property tables.
- 7. Limiting non-composite heights are based on steel properties only.
- 8. Web crippling check based on 1 inch end bearing.
- 9. Shear and web crippling capacity have not been reduced for punchouts.
- 10. Limiting non-composite heights for 48" or 72" o.c. bracing based on properly attached bridging or blocking at the listed spacing.
- Web Stiffeners are required at all support points and concentrated loads.



# **Limiting Wall Heights – Non-Composite**

### Non-Composite Limiting Heights -- Braced at 72" o.c. -- PrimeWall® 20 EQ (19/21mil)

	$F_{y}$	Spacing		5 psf			7.5 psf			10 psf	
Member	(ksi)	o.c. (in)	L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
162PWS134-21	55	12	8' 11"	8' 7"	7' 6"	7' 4"	7' 4"	6' 6"	6' 4"	6' 4"	5' 11"
162PWS134-21	55	16	7' 9"	7' 9"	6' 9"	6' 4"	6' 4"	5' 11"	5' 8"	5' 8"	5' 5"
162PWS134-21	55	24	6' 4"	6' 4"	5' 11"	5' 6"	5' 6"	5' 2"	5' 0"	5' 0"	4' 8"
250PWS134-19	55	12	12' 9"	11'7"	10' 2"	10' 5"	10' 2"	8' 10"	9' 0"	9' 0"	8' 1"
250PWS134-19	55	16	11' 1"	10' 7"	9' 3"	9' 0"	9' 0"	8' 1"	7' 10"	7' 10"	7' 4"
250PWS134-19	55	24	9' 0"	9' 0"	8' 1"	7' 4"	7' 4"	7' 0"	6' 5"	6' 5"	6' 5"
362PWS134-19	55	12	14' 9"	14' 9"	13' 6"	12' 1"	12' 1"	11' 9"	10' 5"	10' 5"	10' 5"
362PWS134-19	55	16	12' 10"	12' 10"	12' 3"	10' 5"	10' 5"	10' 5"	9' 1"	9' 1"	9' 1"
362PWS134-19	55	24	10' 5"	10' 5"	10' 5"	8' 6"	8' 6"	8' 6"	7' 5"	7' 5"	7' 5"
400PWS134-19	55	12	15' 6"	15' 6"	14' 7"	12' 8"	12' 8"	12' 8"	11'0"	11' 0"	11' 0"
400PWS134-19	55	16	13' 5"	13' 5"	13' 3"	11' 0"	11' 0"	11' 0"	9' 6"	9' 6"	9' 6"
400PWS134-19	55	24	11' 0"	11' 0"	11' 0"	8' 11"	8' 11"	8' 11"	7' 9"	7' 9"	7' 9"
600PWS134-21	55	12	17' 3" <sup>e</sup>	17' 3" <sup>e</sup>	17' 3" <sup>e</sup>	14' 1" <sup>e</sup>	14' 1" <sup>e</sup>	14' 1" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>
600PWS134-21	55	16	15' 0" <sup>e</sup>	15' 0" <sup>e</sup>	15' 0" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	10' 7" <sup>e</sup>	10' 7" <sup>e</sup>	10' 7" <sup>e</sup>
600PWS134-21	55	24	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	12' 3" <sup>e</sup>	10' 0" <sup>e</sup>	10' 0" <sup>e</sup>	10' 0" <sup>e</sup>	8' 8" <sup>e</sup>	8' 8" <sup>e</sup>	8' 8" <sup>e</sup>

- 1. Limiting heights are in accordance with AISI S100-16.
- 2. Lateral loads have not been modified for deflection checks.
- 3. Loads or safety factors have not been modified for strength checks.
- 4. 48" and 72" o.c. braced flexural strength taken as the minimum allowable moment based on local buckling, distortional buckling and lateral-torsional buckling with an unbraced length of 48 or 72 inches.
- 5. For distortional buckling allowable moment,  $k\phi = 0$ .
- 6. Moment of inertia for deflection is calculated at the maximum service level stress for the height listed. Note that this value may be higher than the effective lxx listed in section property tables.
- 7. Limiting non-composite heights are based on steel properties only.
- 8. Web crippling check based on 1 inch end bearing.
- 9. Shear and web crippling capacity have not been reduced for punchouts.
- 10. Limiting non-composite heights for 48" or 72" o.c. bracing based on properly attached bridging or blocking at the listed spacing.

Web Stiffeners are required at all support points and concentrated loads.



# **Limiting Wall Heights – Composite**

### Composite Limiting Heights -- PrimeWall® 20 EQ (19/21mil)

	$F_{y}$	Spacing		5 psf			7.5 psf			10 psf	
Member	(ksi)	o.c. (in)	L/120	L/240	L/360	L/120	L/240	L/360	L/120	L/240	L/360
362PWS134-19	55	12	22' 7"	18' 7"	16' 4"	19' 8"	16' 3"	14' 3"	17' 11"	14' 9"	12' 11"
	55	16	20' 6"	16' 11"	14' 10"	17' 11"	14' 9"	12' 11"	16' 2" <sup>†</sup>	13' 5"	11' 9"
	55	24	17' 11"	14' 9"	12' 11"	15' 3" <sup>†</sup>	12' 11"	11' 2"	13' 3" <sup>†</sup>	11' 9"	9' 11"
600PWS134-21	33	12	30' 3"	26' 9"	23' 5"	26' 10"	23' 4"	20' 5"	24' 2" <sup>f</sup>	21' 2"	18' 7"
	33	16	27' 9"	24' 3"	21' 3"	24' 2" <sup>†</sup>	21' 2"	18' 7"	20' 11" <sup>†</sup>	19' 3"	16' 10"
	33	24	24' 2" <sup>1</sup>	21' 2"	18' 7"	19' 9" ¹	18' 6"	16' 3"	17' 1" <sup>†</sup>	16' 10"	14' 7"

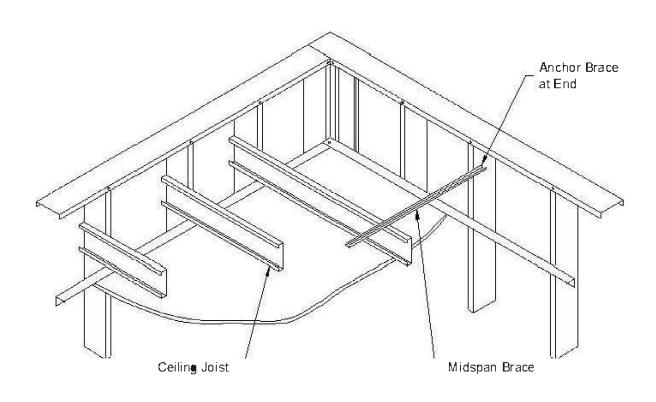
- 1. Allowable composite limiting heights are calculated using ICC-ES AC86-2012.
- 2. The gypsum board must be applied full height to each stud flange and installed using minimum No. 6 Type S Drywall screws spaced a maximum of 12 in. on-center for studs at 24-in spacing, and 16 in. on-center for studs at 16 and 12 in. spacing.
- 3. No fasteners are required for attaching the stud to the track.
- 4. Stud end bearing must be a minimum of 1 inch.

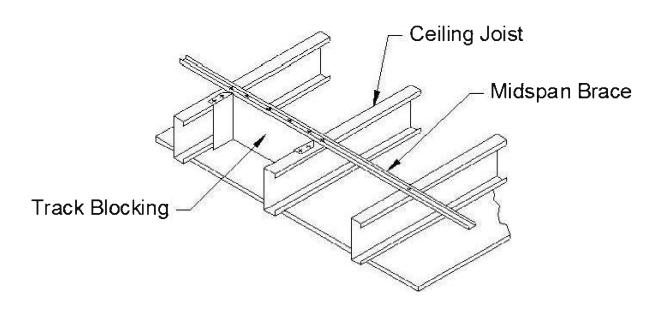
<sup>&</sup>lt;sup>†</sup> Flexural stress controls the allowable wall height.



# **Ceiling Spans**

## **Ceiling Span Tables**







# **Ceiling Spans**

### Allowable Ceiling Spans -- Deflection Limit L/240 -- PrimeWall ® 20 EQ (19/21mil)

				4	psf					6	psf					13	psf		
			Lateral Su	pport of (	Compression	on F <b>l</b> ange			Lateral Su	pport of (	Compressi	on Flange			Lateral S	upport of (	Compressi	on Flange	
	$F_{y}$	U	Insupporte	d		Midspan		U	nsupporte	d		Midspan		U	nsupporte	ed		Midspan	
Section	(ksi)	Joist	Spacing (ii					Joist	Spacing (ii	n) o.c.	Joist :	Spacing (i	n) o.c.	Joist :	Spacing (i	n) o.c.	Joist :	Spacing (ir	1) o.c.
		12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
162PWS134-21	55	8' 5"	7' 9"	6' 10"	9' 3"	8' 4"	7' 4"	7' 5"	6' 10"	6' 0"	8' 0"	7' 4"	6' 4"	5' 11"	5' 5"	4' 9"	6' 2"	5' 7"	4' 10"
250PWS134-19	55	9' 10"	9' 2"	8' 3"	12' 6"	11' 4"	9' 11"	8' 11"	8' 3"	7' 4"	10' 11"	9' 11"	8' 8"	7' 2"	6' 8"	5' 11"	8' 5"	7' 7"	6' 8"
362PWS134-19	55	10' 9"	9' 11"	9' 0"	15' 3"	14' 1"	12' 6"	9' 8"	8' 11"	8' 0"	13' 7"	12' 6"	11' 1"	7' 10"	7' 3"	6' 5"	10' 10"	9' 10"	8' 5" <sup>e</sup>
400PWS134-19	55	11' 0"	10' 2"	9' 2"	15' 8"	14' 5"	12' 10"	9' 11"	9' 2"	8' 3"	13' 11"	12' 10"	11' 5"	8' 1"	7' 5"	6' 7"	11' 2"	10' 2"	8' 9" <sup>e</sup>
600PWS134-21	55	12' 11" <sup>e</sup>	11' 10" <sup>e</sup>	10' 5" <sup>e</sup>	17' 10" <sup>e</sup>	16' 4" <sup>e</sup>	14' 5" <sup>e</sup>	11' 5" <sup>e</sup>	10' 5" <sup>e</sup>	9' 3" <sup>e</sup>	15' 9" <sup>e</sup>	14' 5" <sup>e</sup>	12' 9" <sup>e</sup>	9' 0" <sup>e</sup>	8' 3" <sup>e</sup>	7' 3" <sup>e</sup>	12' 5" <sup>e</sup>	11' 4" <sup>e</sup>	9' 9" <sup>e</sup>

### Allowable Ceiling Spans -- Deflection Limit L/360 -- PrimeWall ® 20 EQ (19/21mil)

				4	psf					6	psf					13	psf		
			Lateral Su	pport of (	Compression	on F <b>l</b> ange			Lateral Su	pport of (	Compressi	on Flange			Lateral S	upport of (	Compressi	on Flange	
	Fy	u	Insupporte	d		Midspan		u	nsupporte	d		Midspan		u	nsupporte	ed		Midspan	
Section	(ksi)	Joist	Spacing (ii	pacing (in) o.c. Joist Spacing (in) o.c. 16 24 12 16 24				Joist :	Spacing (i	n) o.c.	Joist :	Spacing (i	n) o.c.	Joist	Spacing (i	n) o.c.	Joist :	Spacing (ir	1) O.C.
		12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
162PWS134-21	55	8' 1"	7' 4"	6' 5"	8' 1"	7' 4"	6' 5"	7' 0"	6' 5"	5' 7"	7' 0"	6' 5"	5' 7"	5' 5"	4' 11"	4' 4"	5' 5"	4' 11"	4' 4"
250PWS134-19	55	9' 10"	9' 2"	8' 3"	10' 11"	9' 11"	8' 8"	8' 11"	8' 3"	7' 4"	9' 6"	8' 8"	7' 7"	7' 2"	6' 8"	5' 10"	7' 4"	6' 8"	5' 10"
362PWS134-19	55	10' 9"	9' 11"	9' 0"	14' 6"	13' 2"	11' 6"	9' 8"	8' 11"	8' 0"	12' 8"	11' 6"	10' 1"	7' 10"	7' 3"	6' 5"	9' 10"	8' 11"	7' 9" <sup>e</sup>
400PWS134-19	55	11' 0"	10' 2"	9' 2"	15' 8"	14' 3"	12' 5"	9' 11"	9' 2"	8' 3"	13' 8"	12' 5"	10' 10"	8' 1"	7' 5"	6' 7"	10' 7"	9' 7"	8' 4" <sup>e</sup>
600PWS134-21	55	12' 11" <sup>e</sup>	11' 10" <sup>e</sup>	10' 5" <sup>e</sup>	17' 10" <sup>e</sup>	16' 4" <sup>e</sup>	14' 5" <sup>e</sup>	11' 5" <sup>e</sup>	10' 5" <sup>e</sup>	9' 3" <sup>e</sup>	15' 9" <sup>e</sup>	14' 5" <sup>e</sup>	12' 9" <sup>e</sup>	9' 0" <sup>e</sup>	8' 3" <sup>e</sup>	7' 3" <sup>e</sup>	12' 5" <sup>e</sup>	11' 4" <sup>e</sup>	9' 9" <sup>e</sup>

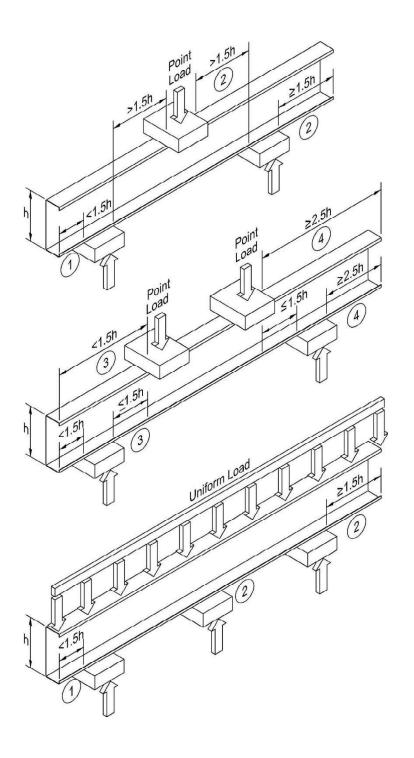
- 1. For unbraced sections, allowable moment is based on AISI S100-16 with KyLy = KtLt = listed span. For mid-span braced sections, allowable moment based on AISI S100-16 with KyLy = KtLt = (listed span)/2.
- 2. For distortional buckling allowable moment,  $k\phi = 0$ .
- 3. Web crippling calculation based on bearing length = 1 inch.
- 4. Web crippling and shear capacity have not been reduced for punchouts. If web punchouts occur near supports members must be checked for reduced shear and web crippling in accordance with AISI S100.
- 5. Safety factors have NOT been multiplied by 0.9 in accordance with AISI S220-15, B1.
- 6. Values are for simple span conditions.
- 7. Moment of inertia for deflection is calculated at the maximum service level stress for the span and load listed. Note that this value may be higher than the effective lxx listed in section property tables.

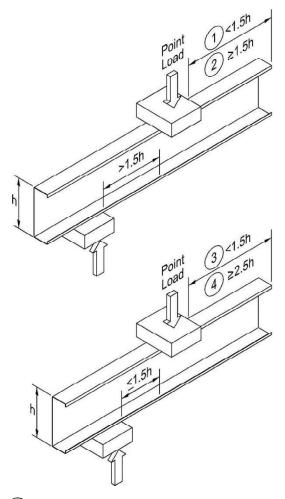
e Web stiffeners required at end reactions.



# **Web Crippling**

### **Web Crippling Conditions**





- Condition 1 End Reaction One Flange
- (2) Condition 2 Interior Reaction One Flange
- (3) Condition 3 End reaction Two Flange
- (4) Condition 4 Interior Reaction Two Flange



# **Web Crippling**

### Allowable Web-Crippling Loads (lbs) -- Single Members -- PrimeWall ® 20 EQ (19/21mil)

			Condition 1					Condition 2						Condition 3					Condition 4				
	Design	Yield F <sub>y</sub>	Bearing Length (in)					Bearing Length (in)						Bearing Length (in)					Bearing Length (in)				
Member	Thickness (in)	(ksi)	1	1.5	3.5	4	6	1	1.5	3.5	4	6	1	1.5	3.5	4	6	1	1.5	3.5	4	6	
162PWS134-21	0.0221	55	126	146	203 <sup>1</sup>	215 <sup>1</sup>	254 <sup>1,2</sup>	210	233	298 <sup>1</sup>	312 <sup>1</sup>	357 <sup>1,2</sup>	108	119	150 <sup>1</sup>	157 <sup>1</sup>	178 <sup>1,2</sup>	286	309	374 <sup>1</sup>	387 <sup>1</sup>	432 <sup>1,2</sup>	
250PWS134-19	0.0200	55	98	114	159	168	199 <sup>1,2</sup>	163	181	234	244	281 <sup>1,2</sup>	71	78	99	103	118 <sup>1,2</sup>	209	226	275	285	319 <sup>1,2</sup>	
362PWS134-20	0.0200	55	92	107	149	158	187 <sup>2</sup>	159	176	227	238	273 <sup>2</sup>	54	60	76	79	90 <sup>2</sup>	186	201	245	253	284 <sup>2</sup>	
400PWS134-21	0.0200	55	90	105	146	155	184 <sup>2</sup>	157	175	226	236	271 <sup>2</sup>	49	55	69	72	82 <sup>2</sup>	179	194	236	244	273 <sup>2</sup>	

#### Notes:

- 1. All capacities listed are calculated using AISI S100-16.
- 2. Web crippling capacities calculated are for studs with stiffened or partially stiffened flanges.
- 3. Tabulated web crippling capabilities are for single members only. For multiple members, multiply the tabulated values by number of members in the assembly.
- 4. Listed allowable capacities are based on members "fastened to supports", except back-to-back members under two-flange loading (conditions 3 and 4) for which data for "fastened to support" is unavailable in the AISI S100-16.
- 5. Listed allowable capacities are for unpunched webs. Capacity reduction for end and interior one flange loading (conditions 1 and 2) near punchouts may be required per Section G6 of S100.

Bearing length to web height ratio, N/h exceeds limit of 2.0 Bearing length to thickness ratio, N/t exceeds limit of 210.



## **Fasteners -- Screws**

### Allowable Screw Connection Capacity (Pounds per Screw) -- PrimeWall ® 20 EQ (19/21mil)

		Min	imum	# 6 Screw				#8 Screw					#1	0 Screw		# 12 Screw			
Steel T	hickness	Steel P	Steel Properties		(0.138" dia; min 5/16" head)			(0.164" dia; min 5/16" head)				(0.190" dia; min 5/16" head)				(0.216" dia; min 5/16" head)			
	Design	Yield	Ultimate	Shear	Pullout	Pullover	Min. Edge	Shear	Pullout	Pullover	Min. Edge	Shear	Pullout	Pullover	Min. Edge	Shear	Pullout	Pullover	Min. Edge
(Mils)	(in)	F <sub>y</sub> (ksi)	F <sub>u</sub> (ksi)	(lbs)	(lbs)	(lbs)	Dist. (in)	(lbs)	(Ibs)	(lbs)	Dist₊ (in)	(lbs)	(lbs)	(lbs)	Dist. (in)	(Ibs)	(lbs)	(lbs)	Dist (in)
19	0.0200	55	55	81	43	172	0.207	88	51	172	0.246	95	59	172	0.285	101	67	172	0.324
21	0.0221	55	55	94	48	190	0.207	102	56	190	0.246	110	65	190	0.285	118	74	190	0.324

- 1. All values assume that the nominal strength of the screw itself is at least 3 times the listed allowable loads.
- 2. Pullover values assume a minimum head/washer diameter, dw, of 5/16" per AISI S100-2016 J4.4.2.
- 3. Minimum Edge distance = 1.5d per AISI S100-2016 J4.2.
- 4. When connecting materials of different thickness or tensile strength, Fu, use the lower values.



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